Paradosso di Nikolai: il momento nonconservativo

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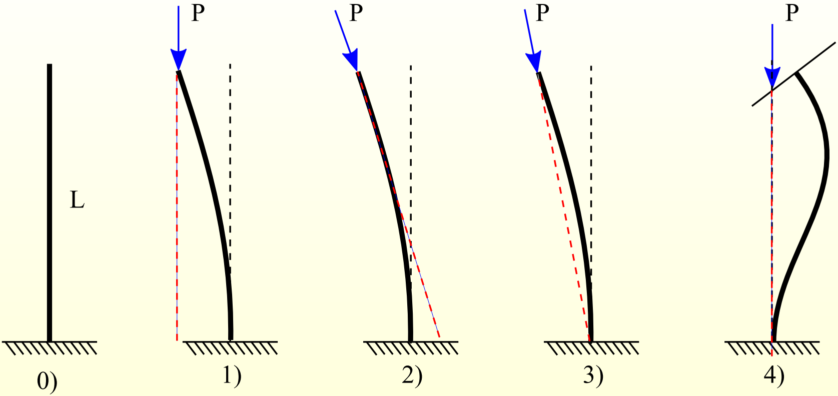
Instabilità di la trave [1]



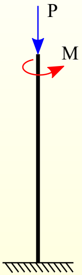
Critical forces: 1) $P^* = \dfrac{\pi^2 EI}{4L^2}$; 2) $P^* = \dfrac{20.5EI}{L^2}$; 3) $P^* = \dfrac{\pi^2 EI}{L^2}$; 4) $P^* = \dfrac{20.5EI}{L^2}$ [Reut, 1939].

[1]Feodosiev, V.I. *Advanced Stress and Stability Analysis. Worked Examples*. Springer, 2005

E. Nikolai paradox

E. Nikolai (1927 – 1930)

Static “solution”



$$EIu'' + Pu = Mv', \quad EIv'' + Pv = -Mu';$$
$$u(s) = C_1 \cos a_1 s + C_2 \sin a_1 s + C_3 \cos a_2 s + C_4 \sin a_2 s,$$
$$v(s) = C_1 \sin a_1 s - C_2 \cos a_1 s + C_3 \sin a_2 s - C_4 \cos a_2 s,$$
$$a^2 + \frac{M}{EI} a - \frac{P}{EI} = 0.$$

Boundary conditions

$$u(0) = v(0) = 0, \quad u'(L) = v'(L) = 0.$$

Solvability $\det \mathbb{A} = 0$ results in

$$\cos \sqrt{\left(\frac{ML}{EI}\right)^2 + \frac{4PL^2}{EI}} = -\left(1 + \frac{M^2}{2PEI}\right)$$

E. Nikolai paradox

E. Nikolai (1927 – 1930)

Solvability condition

$$\cos\sqrt{\left(\frac{ML}{EI}\right)^2 + \frac{4PL^2}{EI}} = -\left(1 + \frac{M^2}{2PEI}\right)$$

has a solution iff $M = 0$

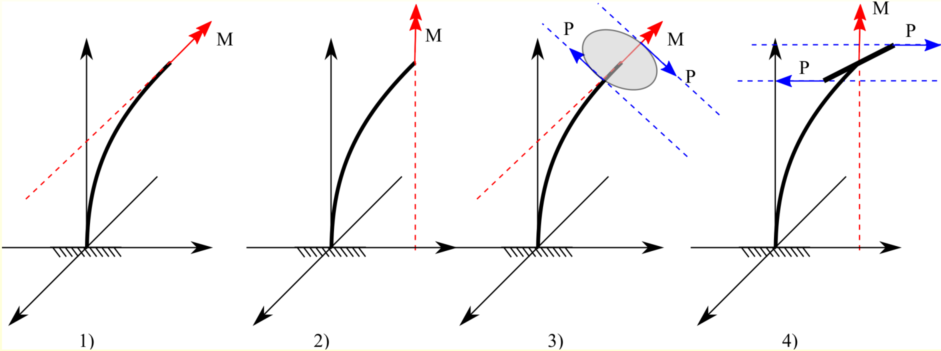
$$\cos\sqrt{\frac{4PL^2}{EI}} = -1, \qquad P^* = \frac{\pi^2 EI}{4L^2}.$$

Conclusion

no equilibrium forms can exist other than the rectilinear form.
Therefore, the applied method is invalid.

Tangential and axial moments (non-conservative)

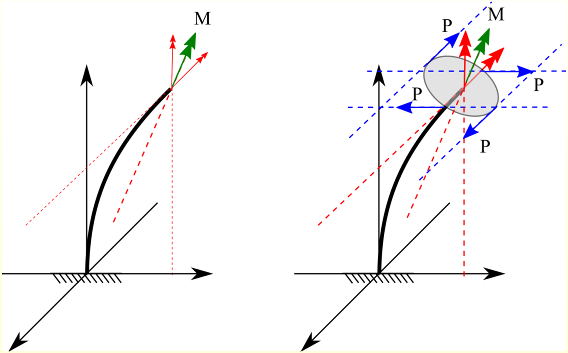
Hans Ziegler introduced: quasi-tangential, semi-tangential and pseudo-tangential moments[2]



Tangential and axial moments are given in 1) and 2) whereas their realizations are given in 3) and 4), respectively.

[2]Bolotin, V. V. *Nonconservative Problems of Theory of Elastic Stability*, Pergamon Press, 1963

Conservative moment



The moment whose vector bisects the angle between the tangent to undeformed and deformed axes of the bar is conservative.

Dynamic analysis

Equations of motion

$$EI_1 \frac{\partial^4 u}{\partial s^4} - M \frac{\partial^3 v}{\partial s^3} + P \frac{\partial^2 u}{\partial s^2} + \rho \frac{\partial^2 u}{\partial t^2} = 0, \qquad (1)$$

$$EI_2 \frac{\partial^4 v}{\partial s^4} + M \frac{\partial^3 u}{\partial s^3} + P \frac{\partial^2 v}{\partial s^2} + \rho \frac{\partial^2 v}{\partial t^2} = 0. \qquad (2)$$

Boundary conditions at $s = 0$

$$u = v = 0, \quad \frac{\partial u}{\partial s} = \frac{\partial v}{\partial s} = 0.$$

At $s = L$ we have

$$EI_1 \frac{\partial^3 u}{\partial s^3} - M \frac{\partial^2 v}{\partial s^2} + P \frac{\partial u}{\partial s} = 0, \qquad (3)$$

$$EI_2 \frac{\partial^3 v}{\partial s^3} + M \frac{\partial^2 u}{\partial s^2} + P \frac{\partial v}{\partial s} = 0, \qquad (4)$$

(transverse forces).

Boundary conditions for different moments

At $s = L$ depending on the loadings we have

- Tangential moment

$$\frac{\partial^2 u}{\partial s^2} = 0, \quad \frac{\partial^2 v}{\partial s^2} = 0.$$

- Axial moment

$$EI_1 \frac{\partial^2 u}{\partial s^2} - M \frac{\partial v}{\partial s} = 0,$$
$$EI_2 \frac{\partial^2 v}{\partial s^2} + M \frac{\partial u}{\partial s} = 0.$$

- Semi-follower moment (pseudo-tangential)

$$EI_1 \frac{\partial^2 u}{\partial s^2} - M \frac{\partial v}{\partial s} = 0,$$
$$\frac{\partial^2 v}{\partial s^2} = 0.$$

Harmonic-type solutions

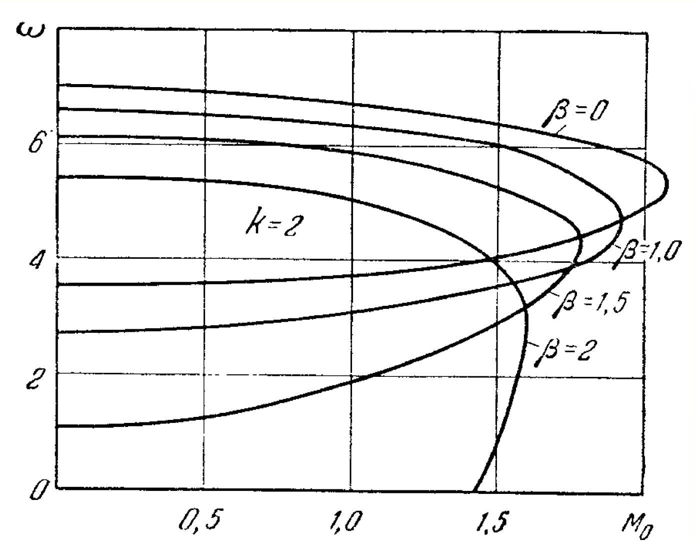
$$u(s,t) = U(s)\exp(i\omega t), \quad v(s,t) = V(s)\exp(i\omega t).$$

Dimensionless parameters

$$M_0 = \frac{ML}{E\sqrt{I_1 I_2}}, \quad \beta^2 = \frac{PL^2}{EI_1}, \quad k = \sqrt{\frac{I_2}{I_1}},$$

$$I_1 \leq I_2, \quad k \geq 1.$$

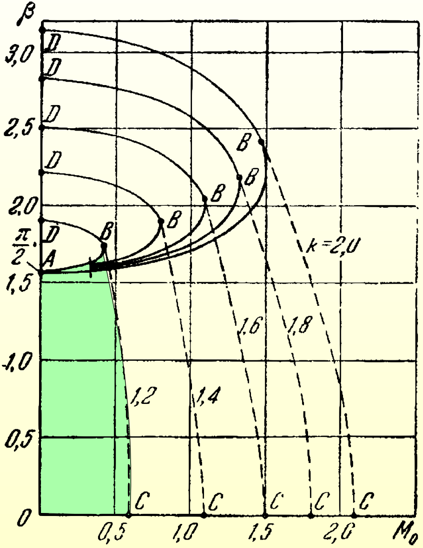
Frequenza ω



Stability zones

Point A:

$$P^* = \frac{\pi^2 EI_1}{4L^2}$$



Feodosiev, V.I. *Advanced Stress and Stability Analysis. Worked Examples*. Springer, 2005

Panovko, I.G. and Gubanova, I.I. *Stability and oscillation of elastic systems: modern concepts, paradoxes and errors*. National Aeronautics and Space Administration, 1973.

Bolotin, V. V. *Nonconservative Problems of Theory of Elastic Stability*, Pergamon Press, 1963.